



2011 Floods and Flood Frequency Analyses

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Flood Frequency Analyses

- What are flood frequency analyses and where do the data come from?
- Why are flood frequency analyses important?
- Why do we use 1% Annual Exceedance Probability instead of 100-year?
- Why are 1% AEP flood values changed?
- Where (and why) are flood data more important to gather?
- Why are gages discontinued?
- Why is 2011 so important to flood frequency analyses?

Flood Frequency Analyses

- Study in cooperation with Montana Department of Transportation and Montana Department of Natural Resources and Conservation
- Update flood frequencies to include data through 2011
- Summarize data in report and provide through Streamstats

2011 Flooding-

“How big was it?” and “Is that big?”



« Previous

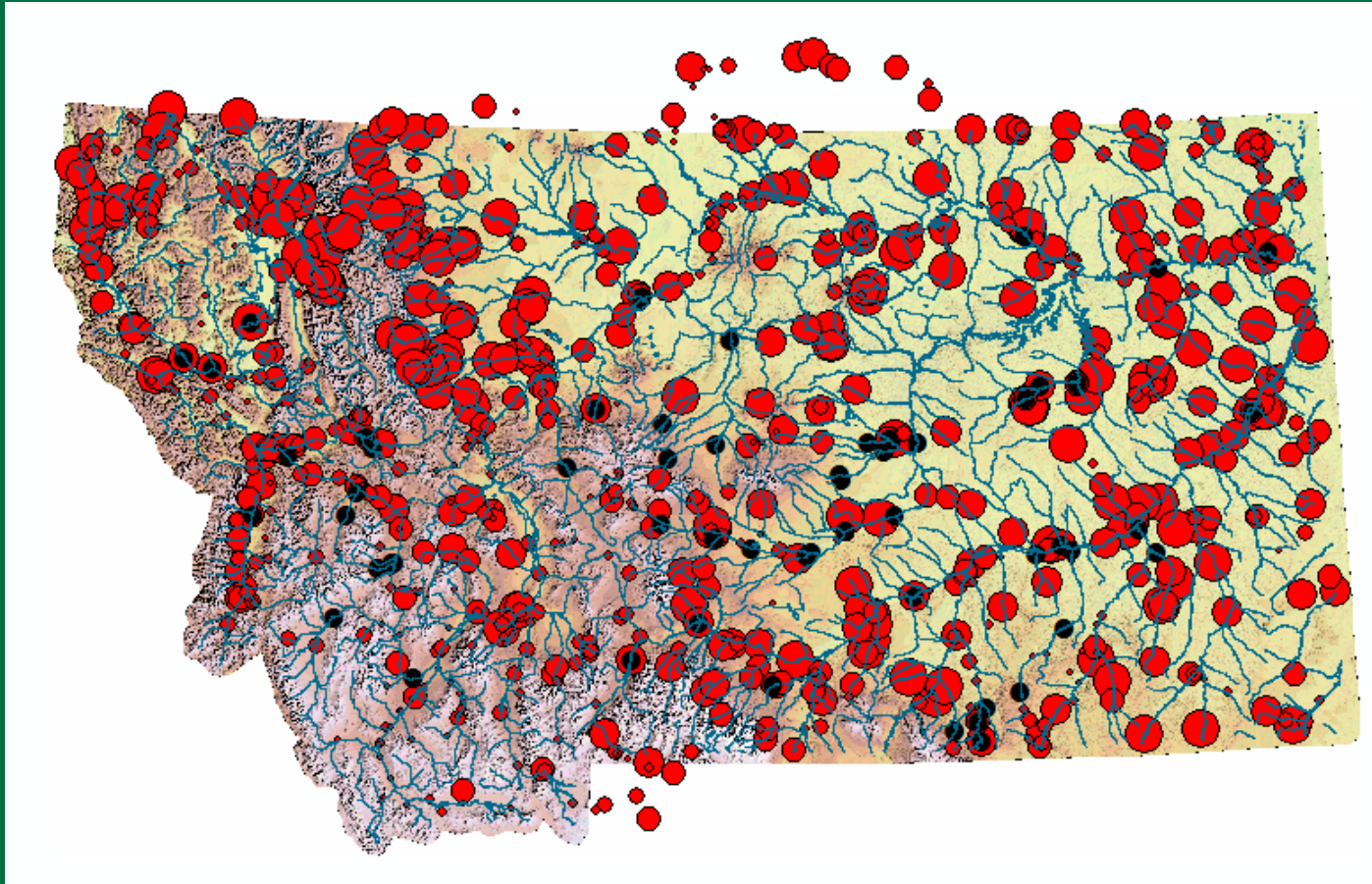
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The Musselshell River floods homes and farms near Martinsdale, Montana

LARRY MAYER\Gazette Staff

2011 Peaks of Record (Discharge, Provisional)



EXPLANATION

- Maximum recorded peak flow normalized to drainage area;
Size of circle is proportional to magnitude
- Stations with maximum recorded peak occurring in 2011, n~54

Musselshell River at Harlowton

- Gaged 1909-Present
- 2011 Peak Discharge ~6,600 cfs

FFA 1909-2009 (102 yrs)

0.4% Flood (~258 yr)

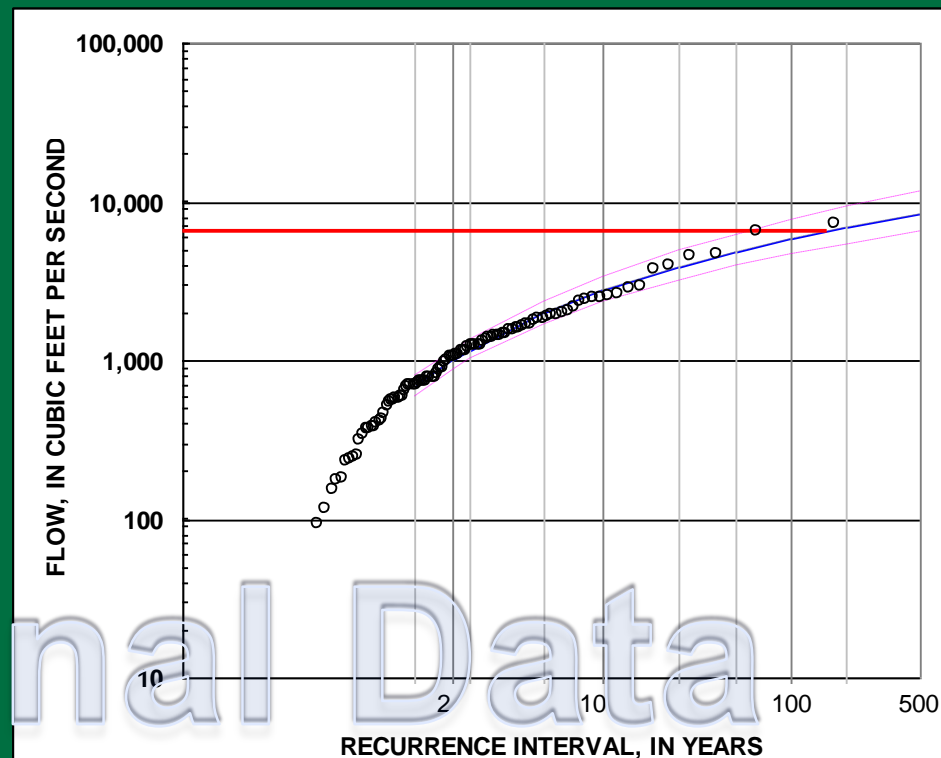
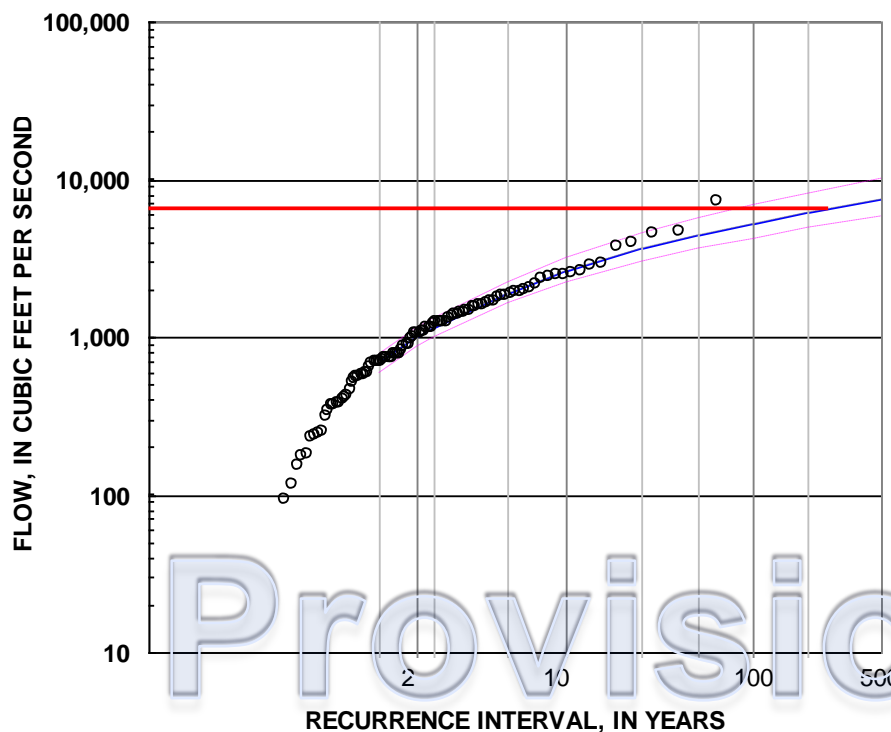
1% Q=5,290 cfs

FFA 1909-2011 (104 yrs)

0.6% Flood (~157 yr)

1% Q=5,840 cfs

10.4%



Musselshell River near Roundup

- Gaged 1946-1948, 1950-Present
- 2011 Peak Discharge ~15,100 cfs

FFA 1946-2009 (63 yrs)

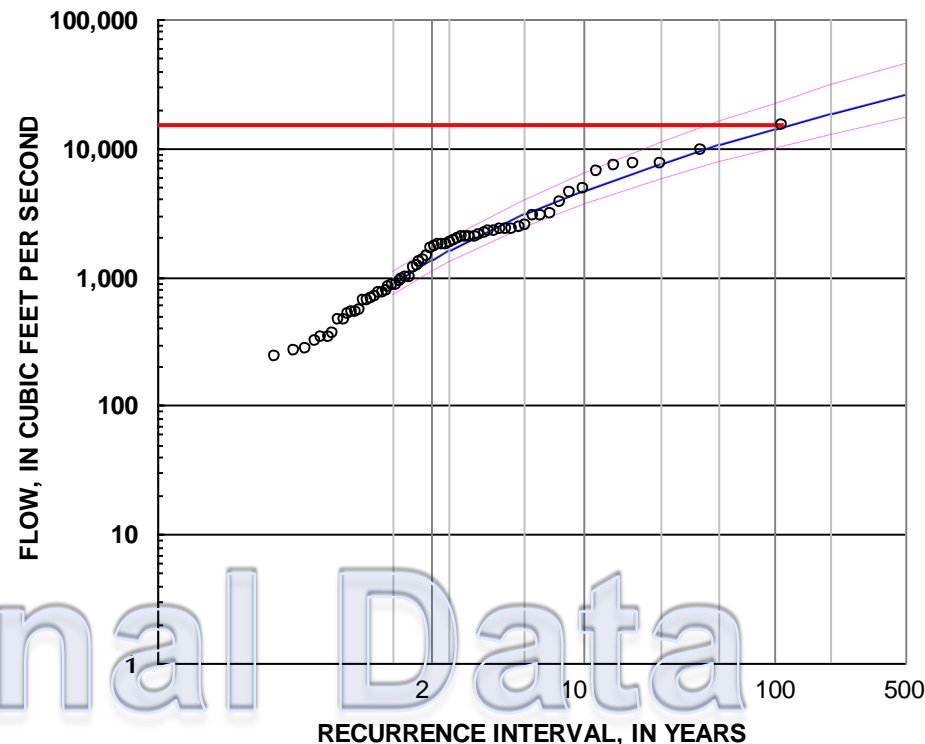
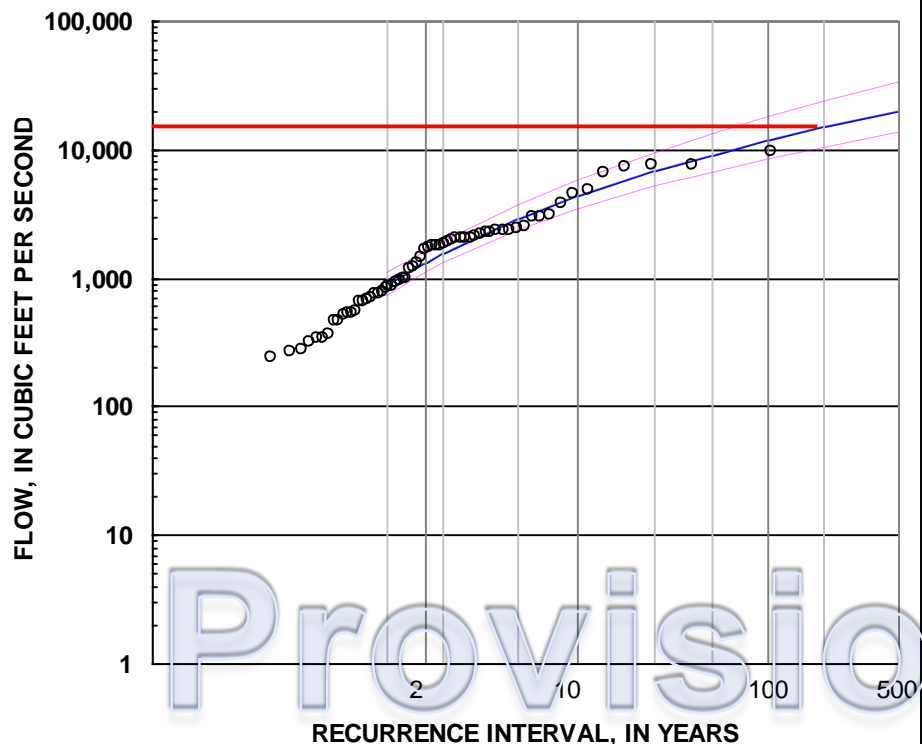
0.54% Flood (~185 yr)

1% Q=11,600 cfs

FFA 1946-2011 (65 yrs)

0.85% Flood (~118 yr)

1% Q=14,190 cfs **22.3%**



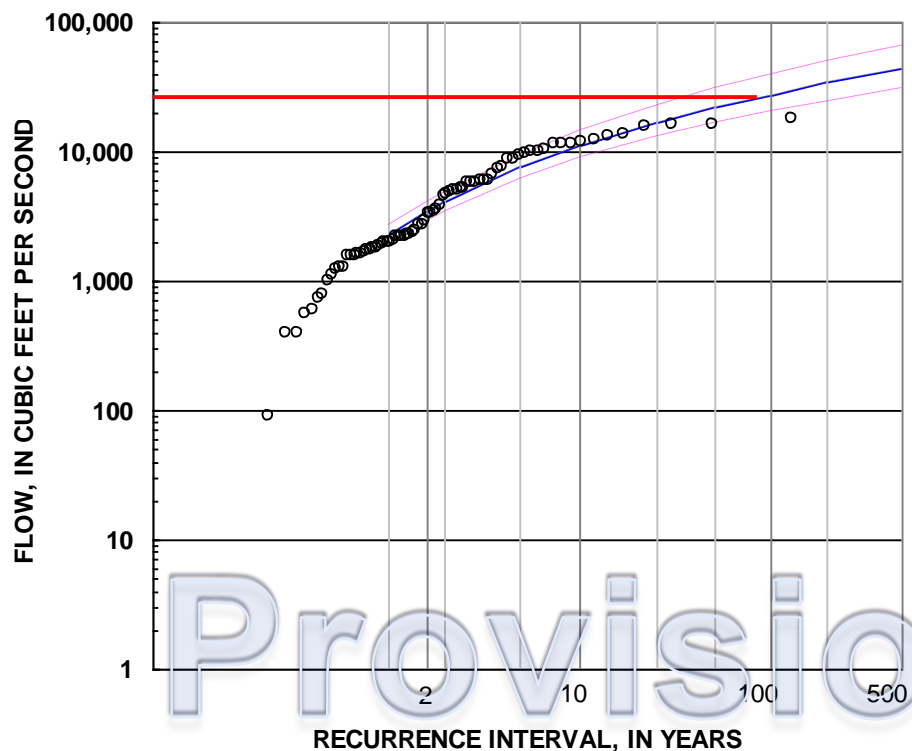
Musselshell River at Mosby

- Gaged 1929, 1932, 1934-46, 1948-Present
- 2011 Peak Discharge ~25,900 cfs

FFA 1929-2009 (78 yrs)

1.2% Flood (~84 yr)

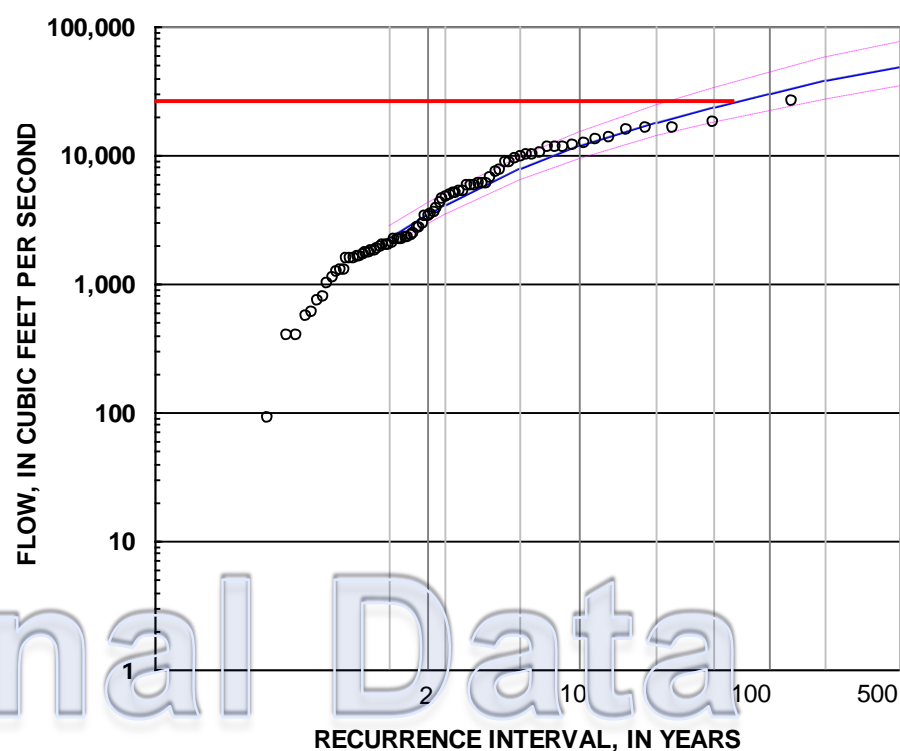
1% Q=27,500 cfs



FFA 1929-2011 (80 yrs)

1.5% Flood (~67 yr)

1% Q=30,300 cfs **10.2%**



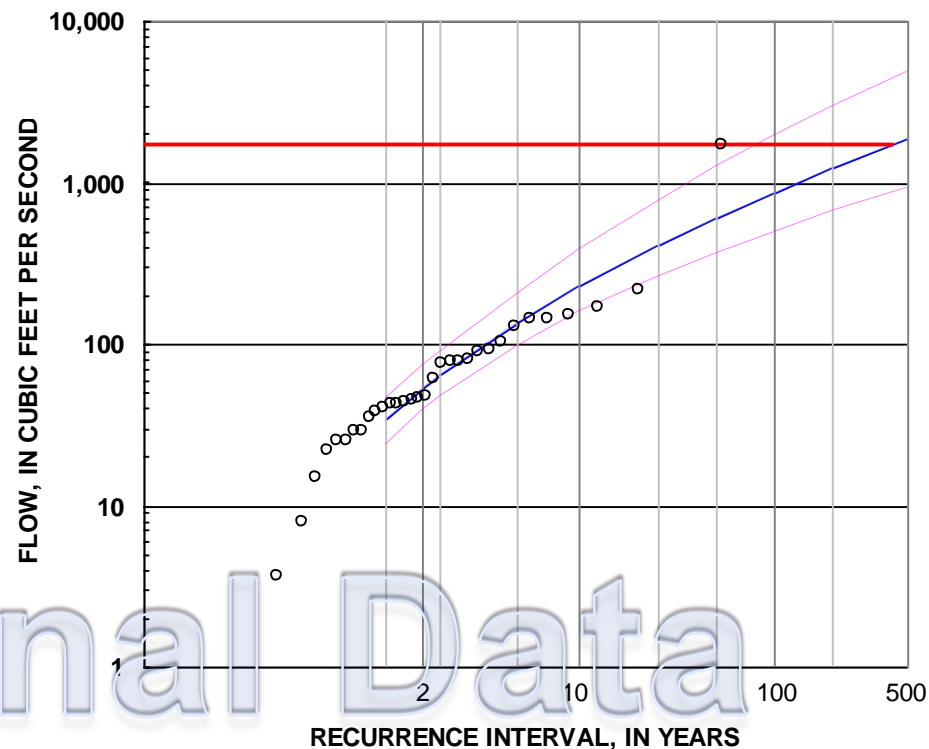
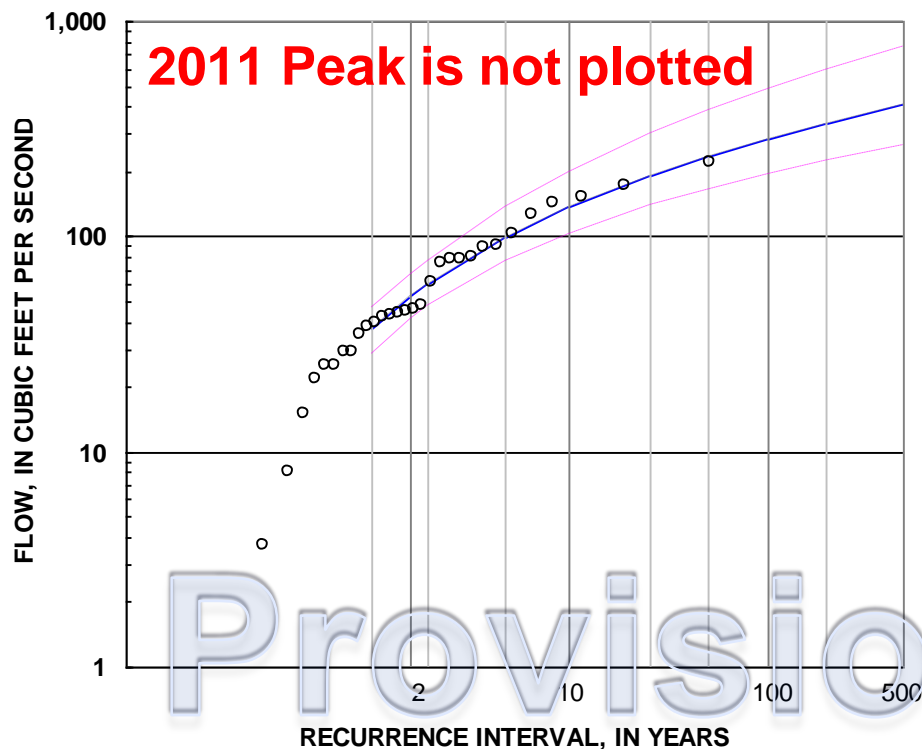
Rosebud Creek near Kirby

- Gaged 1980-Present
- 2011 Peak Discharge ~1,690 cfs

FFA 1980-2009 (30 yrs)
(Frequency not determined)
1% Q=285 cfs

FFA 1980-2011 (32 yrs)
0.24% Flood (~417 yr)
1% Q=870 cfs

205%



Pumpkin Creek near Miles City

- Gaged 1973-78, 1980-85, 2004-Present
- 2011 Peak Discharge ~9,590 cfs

FFA 1973-2009 (18 yrs)

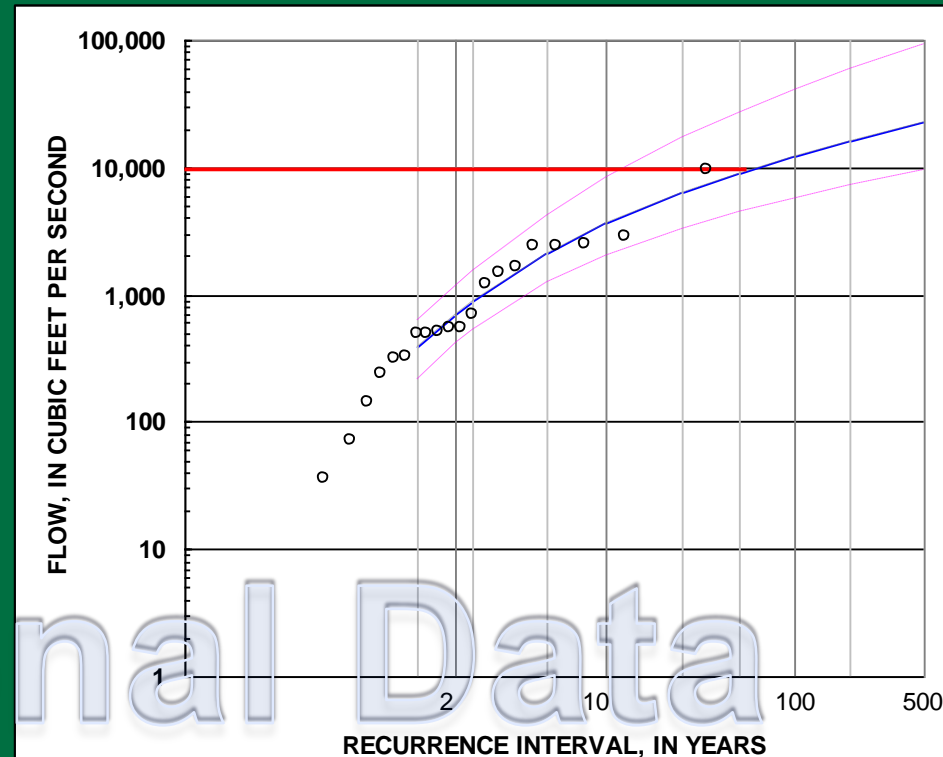
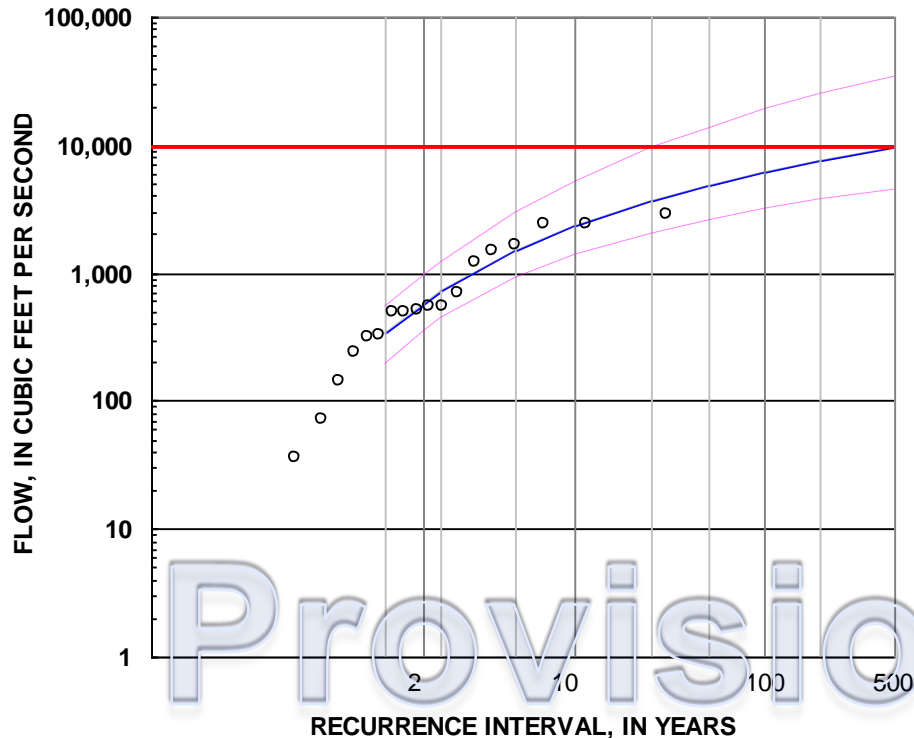
0.2% Flood (~500 yr)

1% Q=6,210 cfs

FFA 1973-2011 (20 yrs)

1.8% Flood (~55 yr)

1% Q=12,320 cfs **98.4%**



Yellowstone River near Livingston

- Gaged 1897-1905, 1929-1932, 1938-Present
- 2011 Peak Discharge ~38,900 cfs

FFA 1897-2009 (86 yrs)

0.68% Flood (~147 yr)

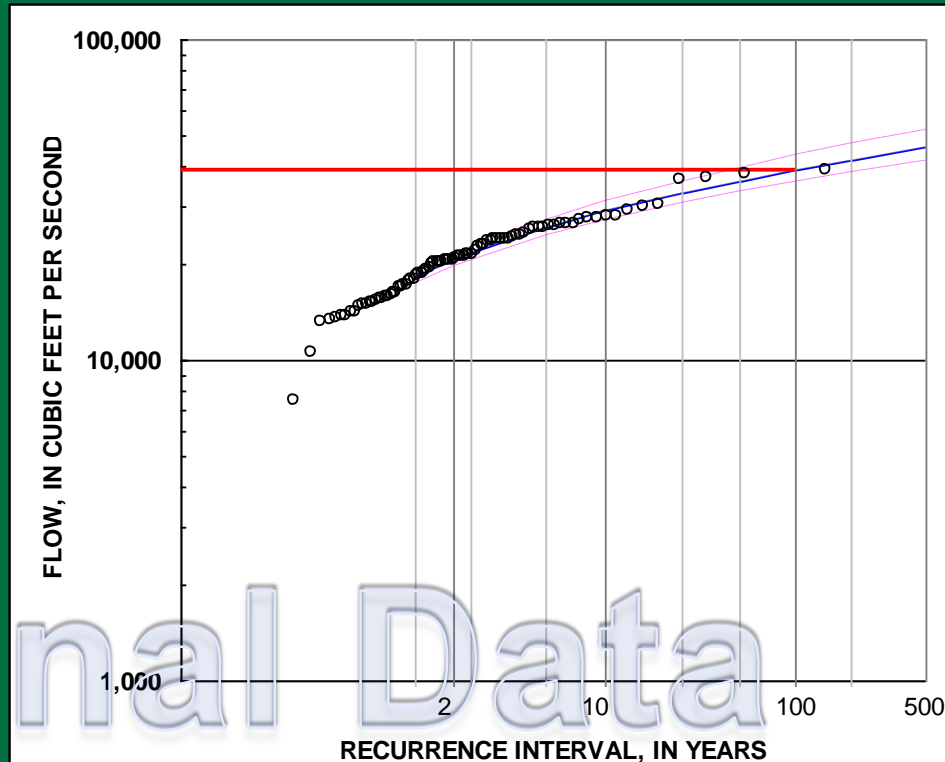
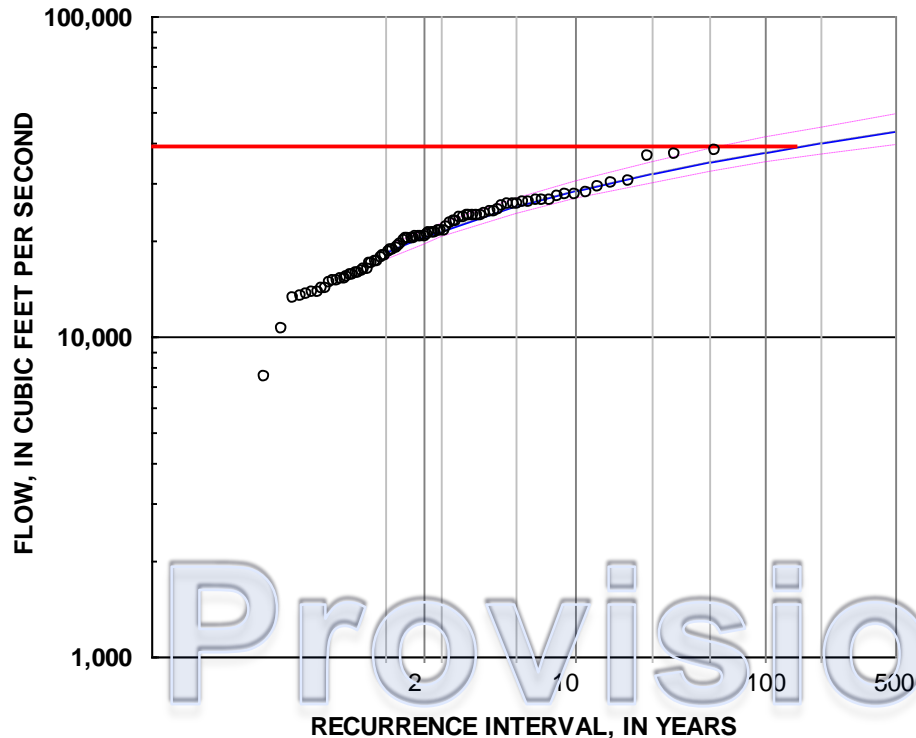
1% Q=37,750 cfs

FFA 1897-2011 (88 yrs)

1.01% Flood (99 yr)

1% Q=39,230 cfs

3.9%



Flatwillow Creek near Flatwillow

- Gaged 1911-32, 1934-36, 1938-1956
- 2011 Peak Discharge ~2,490 cfs

FFA 1911-1956 (44 yrs)

(Frequency not determined; >500 yr)

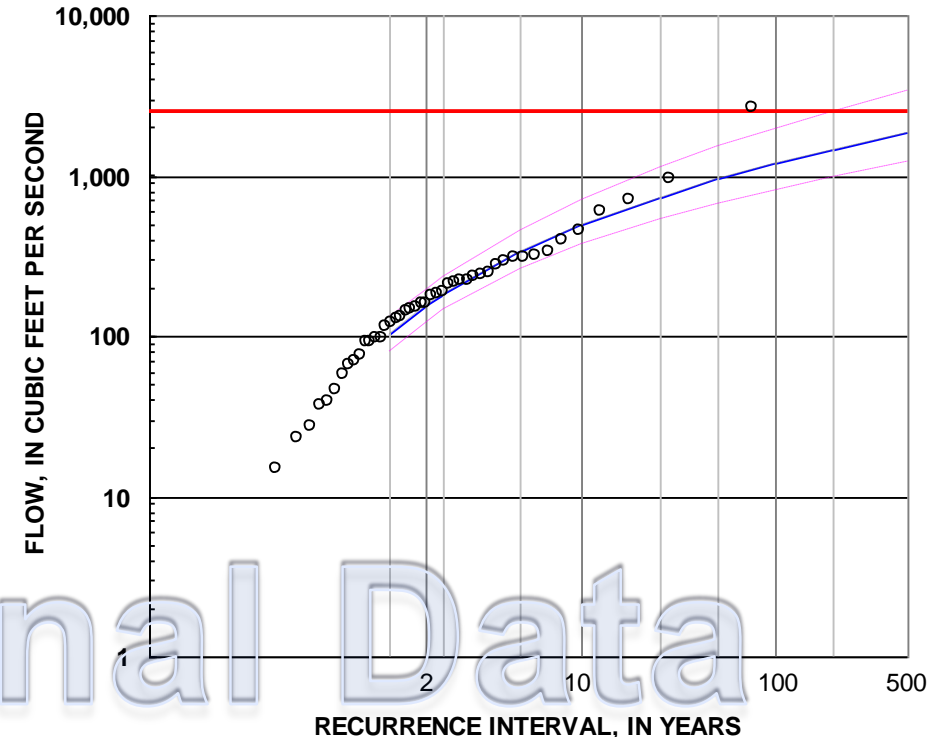
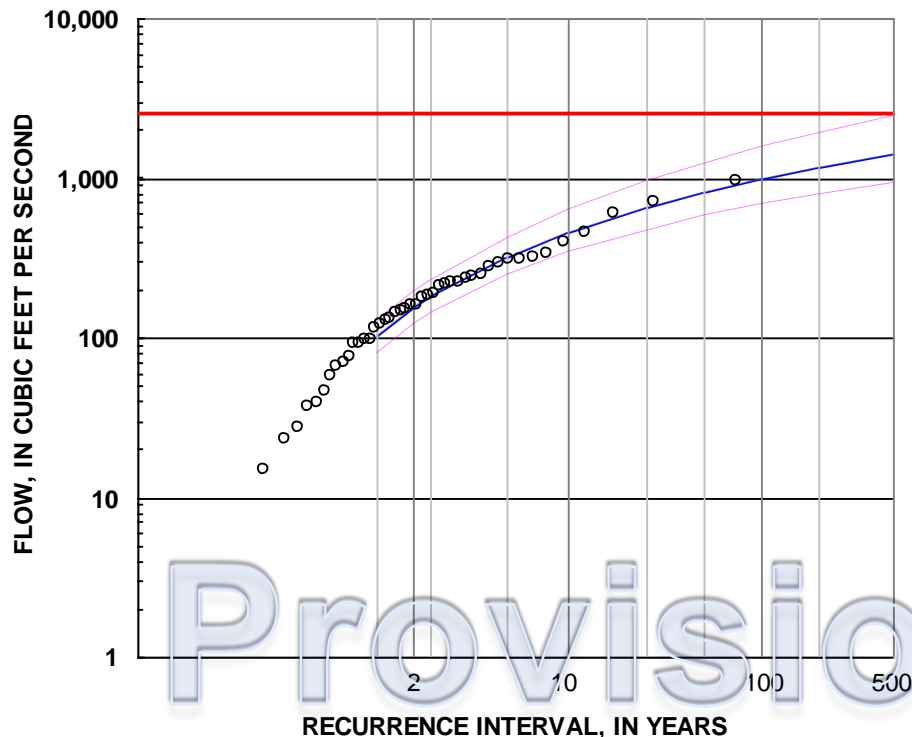
1% Q=973 cfs

FFA 1911-1956, 2011 (45 yrs)

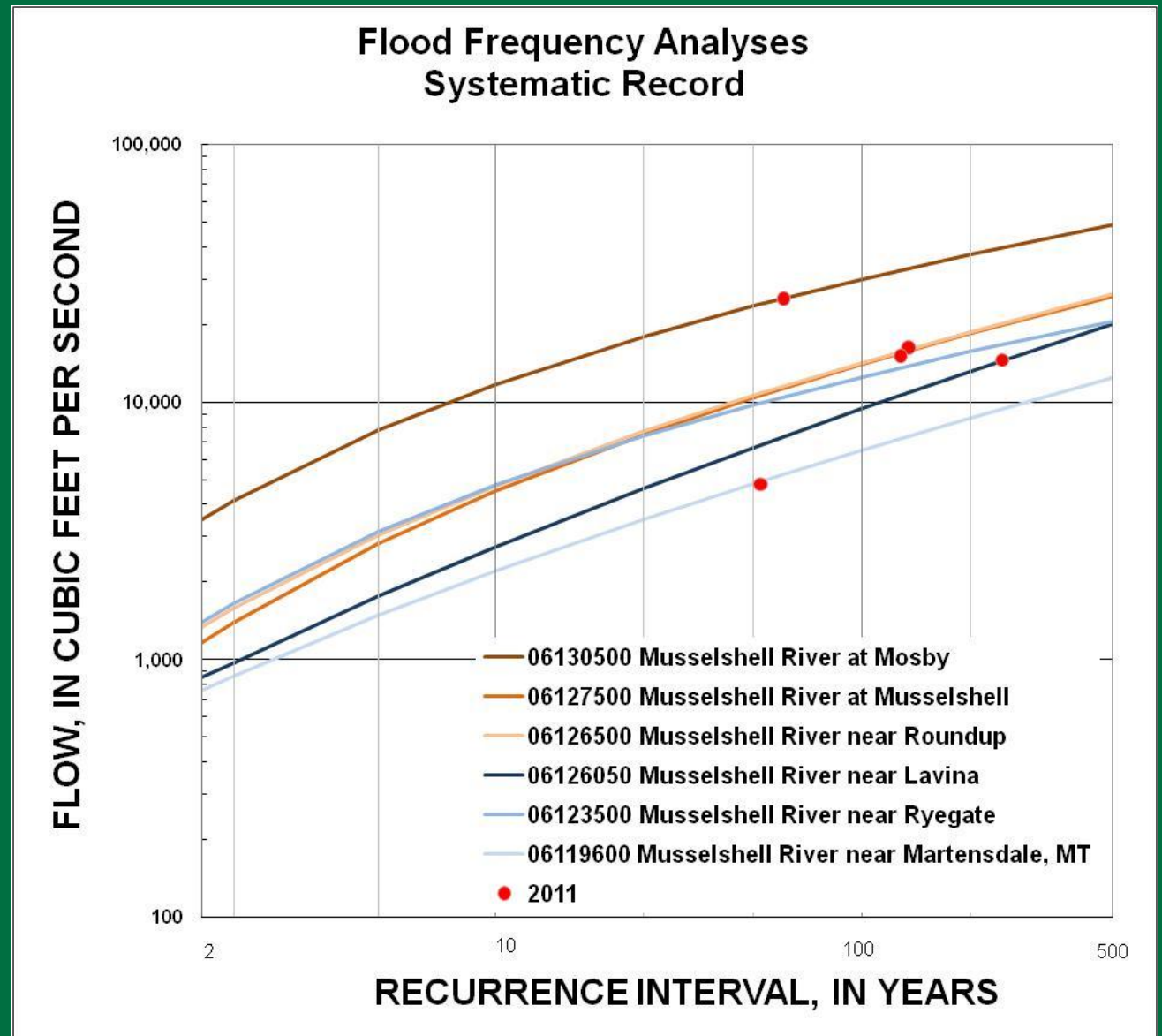
(Frequency not determined; >500 yr)

1% Q=1,200 cfs

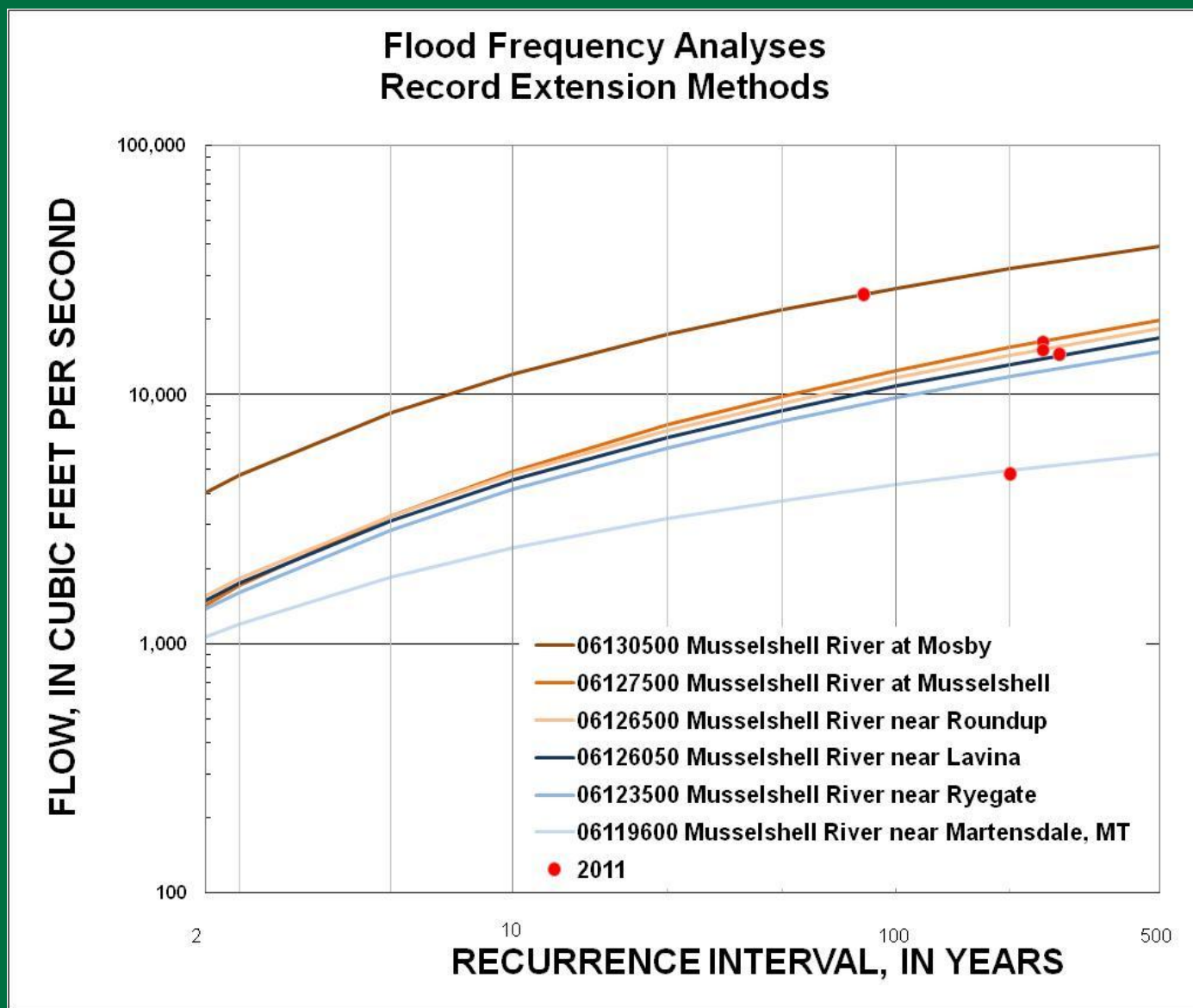
23.3%



Musselshell River-Systematic Record

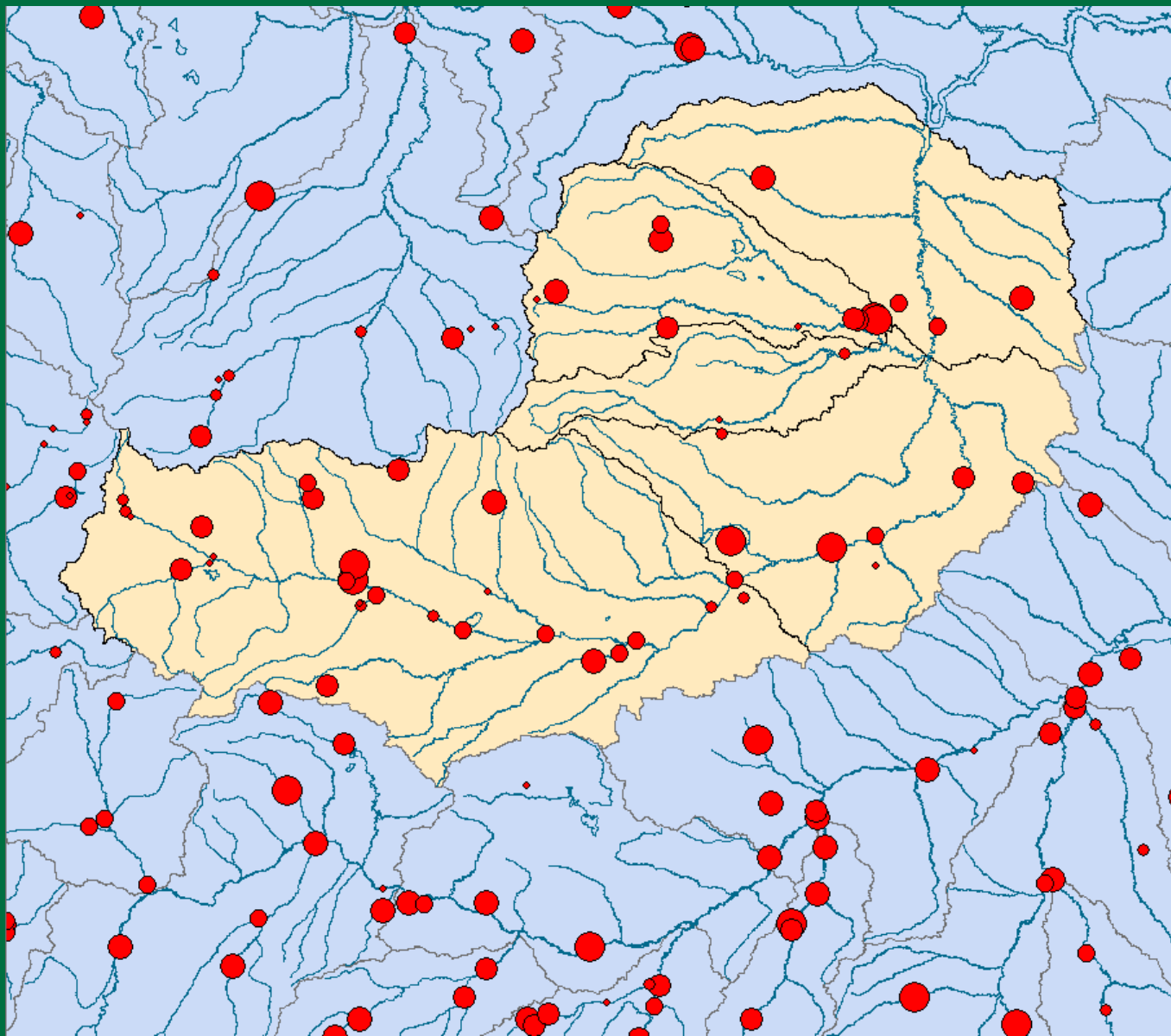


Musselshell River-Record Extension



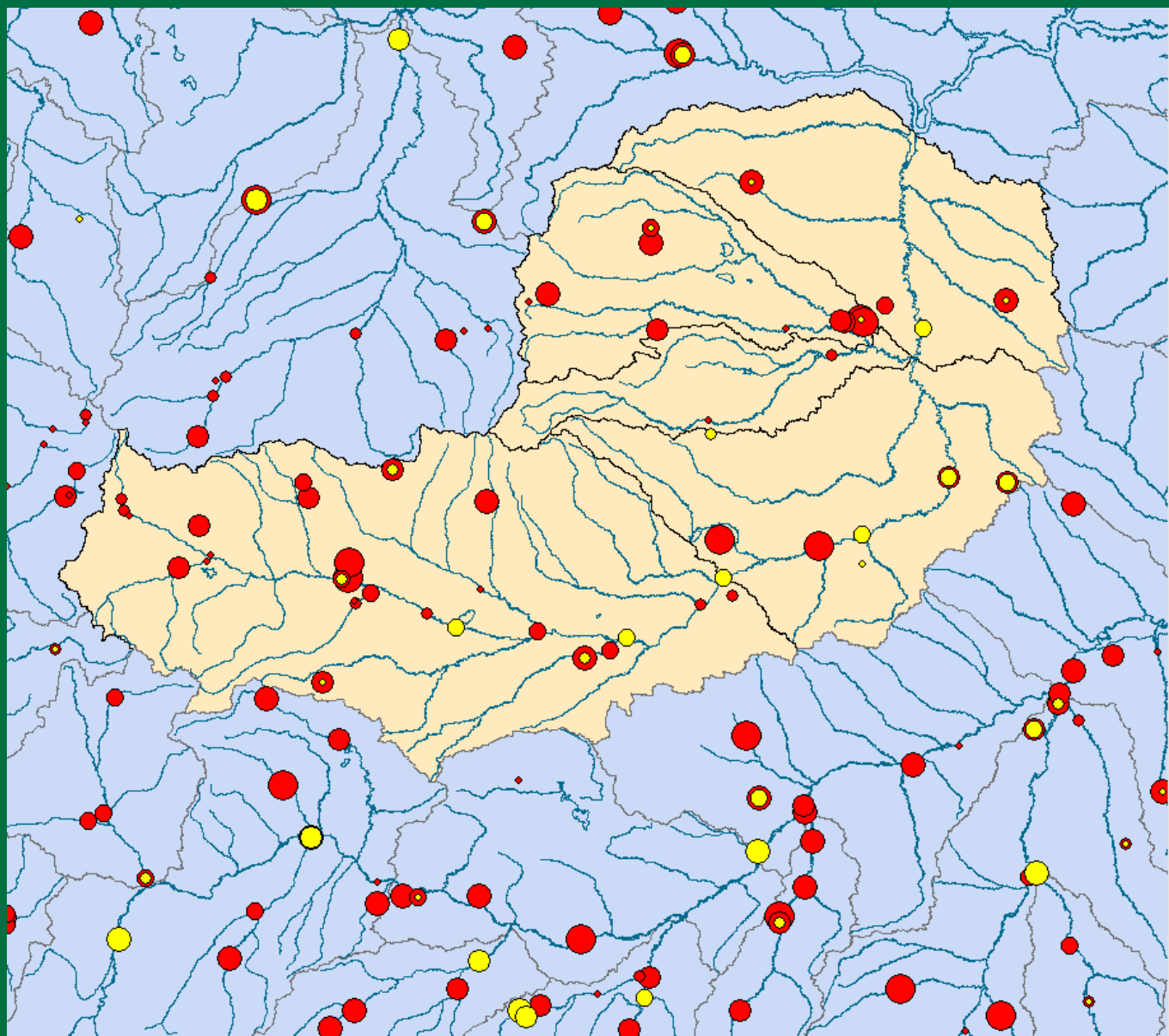
Musselshell Basin-Normalized Peaks

Peaks of record
for all stream
gages (active and
discontinued)



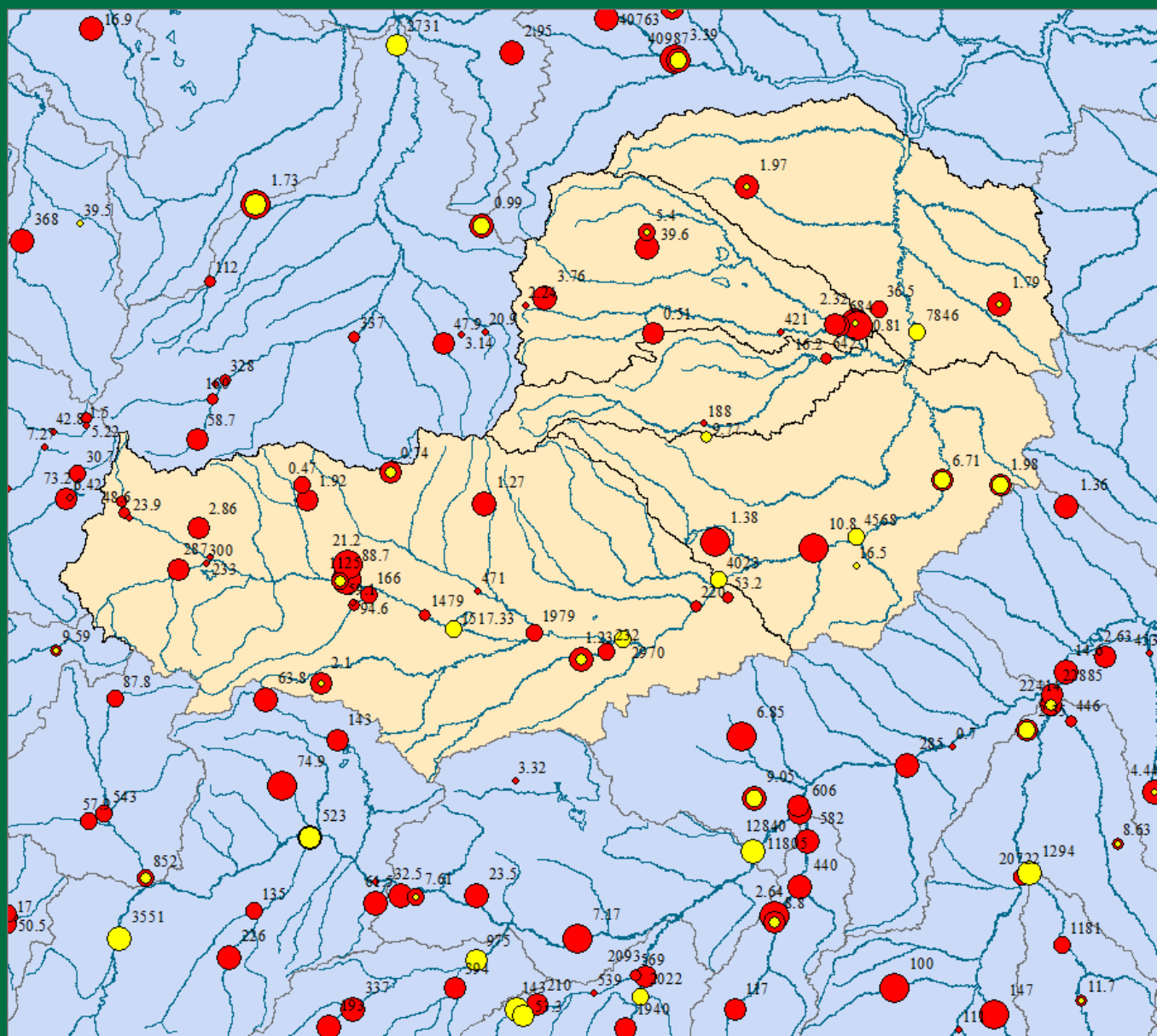
Musselshell Basin-Normalized Peaks

Peaks of record
for all stream
gages (active and
discontinued)
with 2011 peaks
superimposed



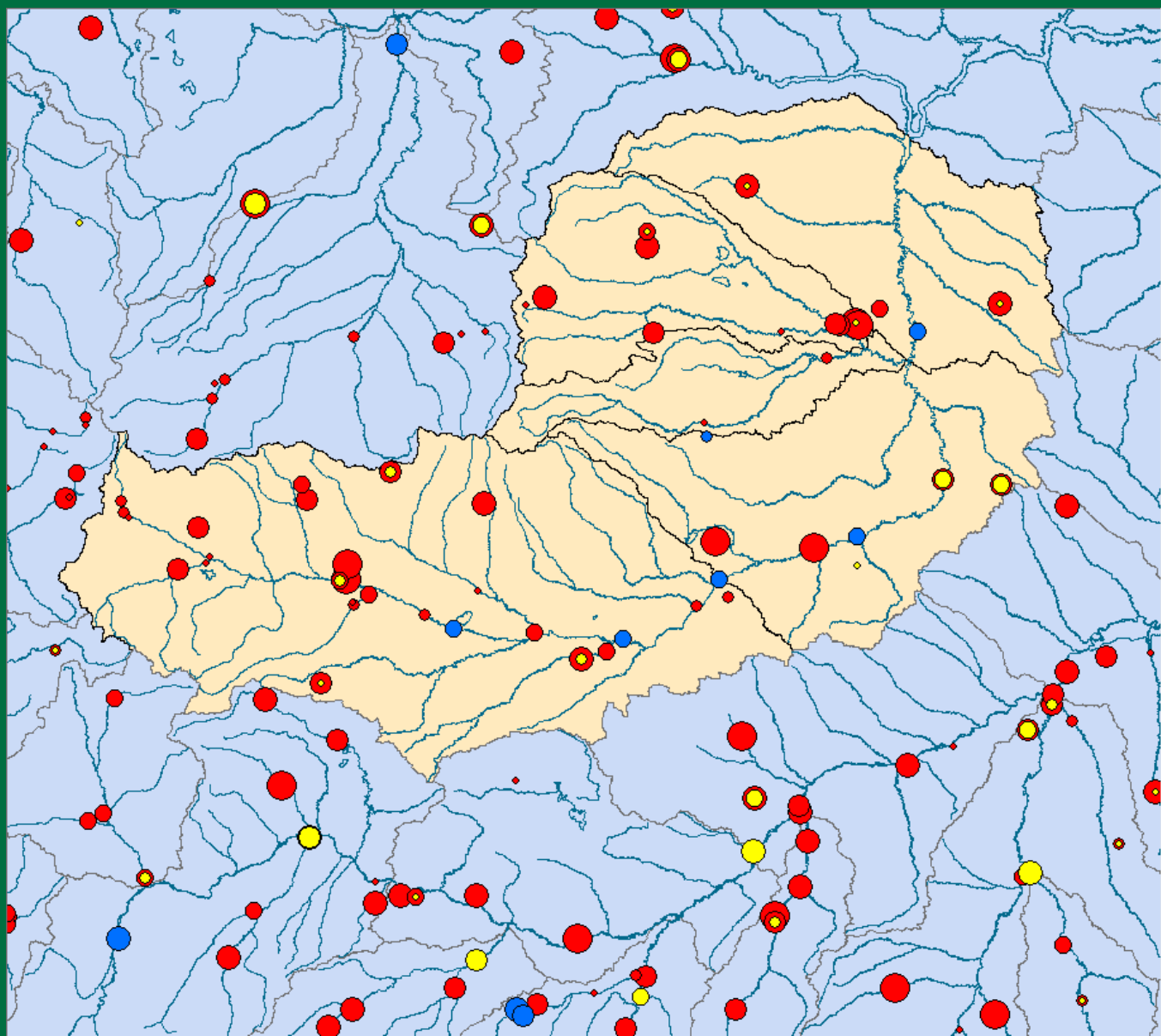
Musselshell Basin-Normalized Peaks

Peaks of record for all stream gages (active and discontinued) with 2011 peaks superimposed; showing drainage areas



Musselshell Basin-Normalized Peaks

Peaks of record
for all stream
gages (active and
discontinued)
with 2011 peaks
superimposed;
2011 peaks of
record in blue



Questions on flood frequency?

Swiftcurrent Creek at Many Glacier, GNP
November 7, 2006



Photograph taken by Don Bischoff, U.S.
Geological Survey, Helena, MT.

Gibson Dam on the Sun River
June, 1964



Photograph taken by George F. Roskie, Lewis
and Clark National Forest



Indirect Discharge Measurements

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U.S. Geological Survey

Data presented in these slides are provisional.

Indirect Measurements

- Computations of peak discharge without using conventional measurement equipment
- Based on high water marks and conservation of energy principles
- Basic types
 - Slope-area
 - Width (bridge) contraction
 - Culvert analysis
 - Road overflow (similar to broad-crested weir)
 - Combination of any or all of the above!

Slope-area



- Uses channel conveyance based on high water marks to compute discharge
- Site data:
 - High water marks (HWMs) along both banks
 - Manning's n values (roughness estimates) for channel and overbanks
 - Minimum 3 cross sections, more is better

Flatwillow Creek near Winnett — (30 Miles north of Roundup)



**Flatwillow
Creek near
Winnett:
Approximate
Cross Section
Locations**



Flatwillow Creek near Winnett



■ $Q = 6,270$ cfs



DISCHARGE COMPUTATIONS							
Reach length (ft)	Discharge (cfs)	Spread (%)	HF (ft)	CX	RC	RX	ER
176.	6024.	0	0.254	1.000	2.146	0.000	#
208.	6501.	0	0.553	1.000	1.531	0.000	@#
230.	6457.	13	0.903	0.934	0.000	-0.228	#
211.	6082.	0	1.205	0.998	0.000	-0.008	@#
384.	6315.	0	0.801	1.000	1.745	0.000	@#
438.	6485.	4	1.461	0.976	0.577	-0.142	@#
441.	6224.	5	2.101	0.975	0.000	-0.096	@
SEC3 - SEC5	2.00						
SEC1 - SEC4	3.00	614.	6352.	3	1.684	0.983	0.840 -0.118 @
SEC2 - SEC5	3.40	649.	6334.	3	2.701	0.985	0.298 -0.077 @
SEC1 - SEC5	4.20	825.	6271.	2	2.923	0.988	0.472 -0.070 @

Definitions:

Spread, the percent difference between discharge computed with no expansion loss ($k=0$) and discharge computed with full expansion loss ($k=1.0$), divided by the discharge computed with full expansion loss

HF, friction head- $HF = \text{sum of } Q^2 * L / (K1 * K2)$ over subreaches; Q, discharge; L, reach length; K1, upstream section conveyance;

K2, downstream section conveyance

CX, the computed discharge divided by the discharge computed with no expansion loss ($k=0$)

RC, velocity head change in contracting section divided by friction head

RX, velocity head change in expanding section divided by friction head

ER, warnings, *-fall < 0.5ft, @-conveyance ratio exceeded, #-reach too short error, 1-negative or 0 fall

*****, terms that can not be computed because of strong expansion in reach

CROSS SECTION PROPERTIES

Width (Bridge) Contraction



- Uses energy loss through bridge
- Site data:
 - I-94 NE of Billings
 - Fall through contraction
 - Substantial contraction of channel width
 - Survey contraction section data (piers, low steel, n values)
 - Approach section data
 - HWMs from approach through contraction

Pryor Creek near Huntley



Pryor Creek near Huntley



■ Q=7,460 cfs

9				Left	Right	Average	Characteristics of constriction			
10	Water-surface elevation at approach section			3031.50	3031.50	3031.50	Sloping embankments; vertical/sloping abutments			
11	Water-surface elevation at contracted section			3028.00	3028.00	3028.00	Left slope	NA	Right slope NA	
12	Fall in Water Surface (delta h) =					3.5	Type of abutment:	Type 2		
13							alpha = summation[(K*3/a^2)]/(Ktotal*3/Atotal^2)			
14	Approach Section - Section 1									
15	Subsection	n	1.486/n	a	w.p.	r	r^2/3	K=(1.486/n)*ar^2/3	K^3/a^2	alpha
16	1	0.043	34.56	313.89	515.18	6.08	3.33	360,896	4,786,055,150	1.00
17										
18	Total			313.89	515.18		K _{app} =	360,896	4,786,055,150	
19										
20	For computing m only									
21	a	0.043	30.96	804.35	116.61	6.90	3.62	90,233	1,135,563,678	1.04
22	q	1.053	28.04	524.87	57.73	9.09	4.36	64,102	956,128,334	
23	b	0.043	30.96	1804.68	340.84	5.29	3.04	169,723	1,501,147,048	
24	Total			3133.89	515.18		K _{app} =	324,058	3,464,981,187	
25										
26	Contracted Section - Section 3									
27	Subsection	n	1.486/n	a	w.p.	r	r^2/3	K=(1.486/n)*ar^2/3	K^3/a^2	alpha
28	1	0.055	27.02	372.97	121.71	3.06	2.11	21,260	69,077,459	n/a
29										
30										
31										
32	Net total (minus piers)				372.97	121.71		K ₃ =	21,260	69,077,459
33										
34	Items and Ratios									
35	delta-h	3.50								
36	bt	55.89	width of the bridge opening/Hv/M	t	0.94	m=1/(Kq/(K _{app}))	0.80			
37	b	55.89	dist. between abutment faces	A ₃ (Gross)	390.99	L/b (left)	1.00 L/b (right)			
38	L (left)	55.89	width of the bridge + x	Y ₁	7.00	r/b	na			
39	L (right)	55.89	width of the bridge + x							
40	Lw	69	Distance from app sec to upstream side of contraction	A _i	18.02	w/b	n/a			
41	r	n/a	radius of entrance	sigma	na	w/b (left)	0.00 w/b (right)			
42	e	0.53	founding	theta	0	(Y _a +Y _b)/2b	0.041			
43	x (left)	0		K _a	90,233	t/(y3-delta-h)	0.0898			
44	x (right)	0								
45	Y _a	1.80	depth flow at embankment	K _b	169,723	See S/W Memo 74.14	F=V ₃ /sqrt(g*Y ₃)	0.60		
46	Y _b	2.75	depth flow at embankment	K _q	64,102		j=A ₃ /A ₁	0.046 (A ₃ incl. piers)		
47	g	32.16	acceleration of gravity	K _{app}	324,058		e=K _a /K _b	0.53		
48	Ld	0	spur length - ** right side only	Ld/b	0.00					
49										
50										
51	C	C*	Kt	Ka*	Kd*	Kj	Ke	Emb slope		
52	0.825	0.86	0.997	*	*	0.962	1	2 to 1	C (left)	
53	0.825	0.86	0.997	*	*	0.962	1	2 to 1	C (right)	
54	* K values included in C* calculation for greater than 2 to 1 slopes									
55	Ccomp =	(C-left*Ka+C-right*Kb)/(Ka+Kb)			=	0.825				

Culvert Computation

- Energy loss through culverts
- Site data:
 - Headwater elevation
 - Tailwater elevation
 - Culvert slope
 - Culvert geometry
 - Culvert roughness
 - Culvert entrance
 - Approach geometry

TYPE	EXAMPLE
1 CRITICAL DEPTH AT INLET $\frac{h_1 - z}{D} < 1.5$ $h_4/h_c < 1.0$ $S_0 > S_c$	$Q = CA_c \sqrt{2g(h_1 - z + a_1 \frac{V_1^2}{2g} - d_c - h_{f1.2})}$
2 CRITICAL DEPTH AT OUTLET $\frac{h_1 - z}{D} < 1.5$ $h_4/h_c < 1.0$ $S_0 < S_c$	$Q = CA_c \sqrt{2g(h_1 + a_1 \frac{V_1^2}{2g} - d_c - h_{f1.2} - h_{f2.3})}$
3 TRANQUIL FLOW THROUGHOUT $\frac{h_1 - z}{D} < 1.5$ $h_4/D \geq 1.0$ $h_3/h_c \geq 1.0$	$Q = CA_3 \sqrt{2g(h_1 + a_1 \frac{V_1^2}{2g} - h_3 - h_{f1.2} - h_{f2.3})}$
TYPE	EXAMPLE
4 SUBMERGED OUTLET $\frac{h_1 - z}{D} > 1.0$ $h_4/D > 1.0$	$Q = CA_0 \sqrt{\frac{2g(h_1 - h_4)}{1 + \frac{29C^2 n^2 L}{R_0^{4/3}}}}$
5 RAPID FLOW AT INLET $\frac{h_1 - z}{D} \geq 1.5$ $h_4/D \geq 1.0$	$Q = CA_0 \sqrt{2g(h_1 - z)}$
6 FULL FLOW FREE OUTFALL $\frac{h_1 - z}{D} \geq 1.5$ $h_4/D \geq 1.0$	$Q = CA_0 \sqrt{2g(h_1 - h_3 - h_{f2.3})}$

Flatwillow Creek near Flatwillow

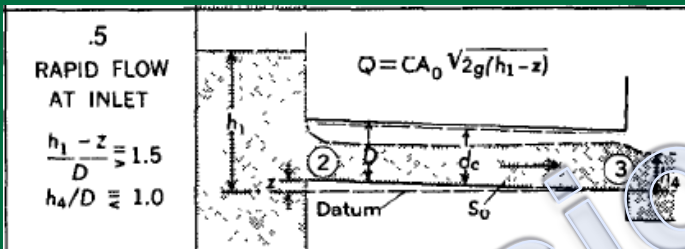
- 2 Culverts
- Road overflow
- Survey data
 - Approach section
 - Culvert geometry and elevation
 - Road centerline and cross section
 - HWMs-
Upstream, downstream,
outside road overflow
influence



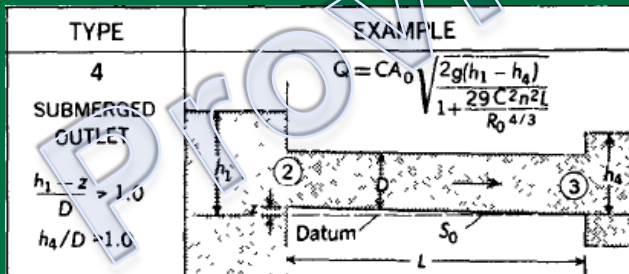
Flatwillow Creek near Flatwillow

■ Culverts

- Culvert analysis program (CAP)
- L. Culvert=635 cfs



- R. Culvert=940 cfs



■ Road Overflow

- Broad crested weir equations
- Road overflow=935 cfs



Need an indirect?

- Surveys
 - Survey early!!!!!!!!!!!!
 - Obvious? - look higher
 - Survey multiple high water marks
 - Pictures and notes
 - Flag marks



Questions?

Swiftcurrent Creek at Many Glacier, GNP
November 7, 2006



Photograph taken by Don Bischoff, U.S.
Geological Survey, Helena, MT.

Gibson Dam on the Sun River
June, 1964



Photograph taken by George F. Roskie, Lewis
and Clark National Forest